#### $S \ E \ C \ T \ I \ O \ N \quad 9 \ 0 \ 5 \ -- \ P \ R \ O \ P \ O \ S \ A \ L \quad (CONTINUED)$

I (We) further propose to execute the attached contract agreement (Section 902) as soon as the work is awarded to me (us), and to begin and complete the work within the time limit(s) provided for in the Specifications and Advertisement. I (We) also propose to execute the attached contract bond (Section 903) in an amount not less than one hundred (100) percent of the total of my (our) part, but also to guarantee the excellence of both workmanship and materials until the work is finally accepted.

I (We) enclose a certified check, cashier's check or bid bond for <u>**five percent (5%) of total bid**</u> and hereby agree that in case of my (our) failure to execute the contract and furnish bond within Ten (10) days after notice of award, the amount of this check (bid bond) will be forfeited to the State of Mississippi as liquidated damages arising out of my (our) failure to execute the contract as proposed. It is understood that in case I am (we are) not awarded the work, the check will be returned as provided in the Specifications.

Bidder acknowledges receipt of and has added to and made a part of the proposal and contract documents the following addendum (addenda):

ADDEN	DUM NO.	1	DATED	7/18/2	011	ADDENDUM NO.	DAT	ED	
ADDEN	DUM NO		DATED			ADDENDUM NO.	DAT	ED	
# E 2 9	640, #2818, idders #3576 , 907-512-1,	and #303 ; Add Spec 907-701-4, d 907-804-	s; Add Notice to 39; Revised No cial Provisions 9 907-711-4, 90 13; Revised Bio	otice to 907-110- 7-713-2,	(Must a	L ADDENDA:1 agree with total addence etfully Submitted,	la issued prior to	opening of	bids)
							Contractor		
					BY				
					<u> </u>		Signature		·
					TITLE				
					ADDR	ESS			
					CITY, STATE, ZIP				
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(To be filled	l in if a corp	oration)							
			red under the e executives an			of		and	the names,
	Pres	ident					Address		
	Seci	retary					Address		
	Trea	surer					Address		
The following	ng is my (ou	r) itemize	d proposal.				400450004		
Revised 09/21/2005				NH-0007-01(076) / 106159301 Marshall Cou			County(ies)		

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SECTION 905 - PROPOSAL,

PROPOSAL BID SHEETS,

COMBINATION BID PROPOSAL,

CERTIFICATION OF PERFORMANCE - PRIOR FEDERAL-AID CONTRACTS, CERTIFICATION REGARDING NON-COLLUSION, DEBARMENT AND SUSPENSION, SECTION 902- CONTRACT FORM, AND SECTION 903 - CONTRACT BOND FORMS, FORM -- OCR-485.

#### (REVISIONS TO THE ABOVE WILL BE INDICATED ON THE SECOND SHEET OF SECTION 905 AS ADDENDA)

#### SECTION 904 - NOTICE TO BIDDERS NO. 640

CODE: (IS)

DATE: 09/26/2005

#### **SUBJECT:** Fiber Reinforced Concrete

Bidders are hereby advised that synthetic structural fibers meeting the requirements of Subsection 907-711.04 may be used in lieu of wire mesh in some items of construction. Substitution of fibers for wire mesh will be allowed in the construction of paved ditches, paved flumes, paved inlet apron, driveways, guard rail anchors and pile encasements. Substitution in any other items of work must be approved by the State Construction Engineer prior to use.

#### **SECTION 904 - NOTICE TO BIDDERS NO. 2818**

CODE: (SP)

#### DATE: 10/01/2009

#### SUBJECT: Non-Quality Control / Quality Assurance Concrete

Bidders are advised that the following pay items will not be accepted based on the Quality Control / Quality Assurance (QC/QA) requirements of Section 804 of the specifications. The acceptance of these pay items will be based on sampling and testing at the project site by MDOT forces. The Contractor is required to submit mix designs to accomplish this work in accordance with Section 804 and perform normal Quality Control functions at the concrete plant. Acceptance will be in accordance with the requirements of 907-601, Structural Concrete, and TMD-20-04-00-000. At the discretion of the Engineer, the Contractor may request that the concrete be accepted based on QC/QA requirements.

Pay Item	<b>Description</b>
221	Paved Ditches
601	Minor Structures - manholes, inlets, catch basins, junction boxes, pipe headwalls, and pipe collars.
606	Guardrail Anchors
607	Fence Post Footings
608	Sidewalks
609	Curb and Gutter
614	Driveways
616	Median and Island Pavement
630	Sign Footings, except Overhead Sign Supports

#### **SECTION 904 - NOTICE TO BIDDERS NO. 3039**

CODE: (SP)

DATE: 03/23/2010

#### SUBJECT: Alternate Asphalt Mixture Bid Items

Bidders are advised that the asphalt mixture used on this project will be bid as an alternate pay item: Hot Mix Asphalt (HMA) or Warm Mix Asphalt (WMA). Bidders must select one of the alternates at the time of bid. The Contractor must use the selected asphalt mixture, HMA or WMA, throughout the entire project.

#### SECTION 904 - NOTICE TO BIDDERS NO. 3576

CODE: (SP)

DATE: July 15, 2011

SUBJECT: SCOPE OF WORK

#### **PROJECT:** NH-0007-01(076) / 106159301 -- Marshall County

The contract documents do not include an official set of construction plans but may, by reference, include some Standard Drawings when so specified in a Notice to Bidders entitled, "Standard Drawings". All other references to plans in the contract documents and Standard Specifications for Road and Bridge Construction are to be disregarded.

The work is to be accomplished using the Pay Items and corresponding specifications set forth in this contract: various milling, trench widening and overlaying on approximately twelve (12) miles of US Highway No.72, both eastbound and westbound beginning at the beginning of the four lane section and ending at the Benton County Line, in Marshall County.

Bidders are advised that cross-slopes for curve superelevations are to be constructed in accordance with information provided by the Department. To assist the contractor in correctly placing the cross slope transitions, the Department will provide at the preconstruction conference the stationing and percent slope information. Sufficient preleveling and leveling quantities are provided in this contract to facilitate the work. All transition cross slopes are to be established and checked prior to placement of the final lift of pavement.

It shall be the responsibility of the contractor to protect the roadway and all existing structures, such as bridges and curb, from damage occurring as a result of the contractor's operations. Damages to existing features caused by the contractor's operations shall be repaired or replaced at no cost to the Mississippi Department of Transportation.

At bridge ends and at the end of work day, a taper of one (1) vertical inch for each three (3) horizontal foot shall be provided.

The contractor shall make a Mississippi One-Call request prior to any excavation, excepting trench widening or pavement removal/repair.

In order to expedite the safe movement of traffic and to protect each phase of the work as it is performed, a firm sequence of operations is essential. The work shall be begun and continually prosecuted.

The work on this section shall consist of the following:

1. Pressure grouting of the approach slabs as noted in the table at the conclusion of this Notice to Bidders. Actual quantities may vary in the field based on the depth of the void present. This work will be completed using the following:

512-A, Holes, 907-512-B, Portland Cement Pressure Grout Slurry, Type 1 512-C, Calcium Chloride

2. Sealing of expansion joints per the table at the conclusion of this Notice to Bidders. Actual quantities may vary in the field based on the actual dimensions of the bridge. The removal of existing expansion joint sealant material and surface preparation prior to the placement of the poured joint sealant is to be completed in accordance with Contract Specifications. Any and all work necessary to remove any existing material and prepare the joint to receive the poured joint material will be absorbed items, and is to be included in the cost bid, using pay item 508-A001, Silicone Sealed Joints. The placement of the expansion joint material will be in accordance with the contract specifications and Standard Drawing Sheet BE-1, which is included in this Notice to Bidders.

3. Cold milling of the existing asphalt pavement to a depth of one and one-half (1 1/2) inches and variable, removing the top lift of asphalt, at the transition areas at both the BOP and EOP and at local roads as shown in the attached table. The cold milling material obtained shall become the property of the contractor. The mainline roadway will not be milled other than as shown in the attached table.

Payment for Cold Milling of Pavement will be made under Pay Item no. 406-A, per square yard, and shall include all cost associated with the milling operation.

NOTE: During this operation and prior to placement of the asphalt, due care shall be required to keep surface water from ponding on the roadway surface; continuous monitoring of the project may be required.

4. Excavate three (3) inches deep and two (2) feet wide at the pavement edges for trench widening for a finished pavement width of twenty-eight (28) feet. The remaining shoulder width will require placement of granular material. A portion of the mainline has already been widened and will not require additional widening. Some excavation will be required in order to extend the entrance at three county roads, which are not currently paved.

NOTE: Prior to this excavation, sufficient granular material must be in place to provide an acceptable trench widening operation.

NOTE: Any material removed from the excavation operation of trench widening that cannot be reasonably used as part of the final shoulder shall be removed; and this material shall be moved simultaneous with the trench widening operation. Neither shall this material be allowed to remain rolled up on the existing shoulder or bladed down the existing slope. There will be no separate payment for this work which shall be included in the pay items provided.

NOTE: The trench widening excavation may include the removal of some asphalt along the shoulder due to previous maintenance repairs.

NOTE: Due care shall be taken during this operation to blade material to the roadway and away from the ditch line. Material inadvertently bladed to the roadway vegetation shall be removed at no cost to the Mississippi Department of Transportation.

5. Placement of three (3) inches of trench widening (ST, 19 mm, trench widening) in the previous excavation. Approximately 200 tons of ST 19 mm trench widening mix is also provided for placement at three existing county roads which are not paved.

- 3 -

6. Placement of a "scratch course" of Hot Mix Asphalt mix (HT, 4.75 mm) for rut filling and to establish a uniform cross section and an acceptable centerline profile prior to placement of the leveling course, as directed by the Engineer. This shall be accomplished so that only the existing ruts are filled with Hot Mix Asphalt. This course is not intended to be full width of the roadway. It will consist of overlaying four (4) lanes of traffic on mainline between the stripe. Any stripe damaged by this initial "scratch course" shall be repaired at no additional cost to the Mississippi Department of Transportation. The "scratch course" shall not be placed on the ramps, county roads, crossovers, or the trench-widened areas. The course shall be "Rolled to Refusal." Compaction for this course shall be accomplished with a pneumatic roller. Steel wheel rollers shall not be used. The purpose of this "scratch course" is to establish a plane surface for a uniform thickness on the next course, a one and one-half  $(1 \frac{1}{2})$  inch overlay.

#### <u>Use of a rubber tire roller will be required in rutted areas to insure proper compaction in wheel paths.</u>

 Repair failed areas using the following: 503-C, Saw Cut, Full Depth 202-B, Removal of Pavement, All Types and Thicknesses—for pavement structure 203-G, Excess Excavation –for material below the pavement structure 907-403-C, Hot Mix Asphalt, ST, 19 mm, Trench Widening to replace failed areas

NOTE: Replacement shall be made same day as excavation.

8. Placement of one and one-half inch  $(1 \frac{1}{2} )$  of surface course (HT, 9.5 POLYMER Modified) on the twenty-eight (28) feet of pavement previously "scratched" and crossovers. The ramps at SR 302 are to be paved one and one-half inch  $(1 \frac{1}{2})$  of surface course (HT, 9.5 mm POLYMER Modified) to the point that the ramp/loop returns to a uniform or standard width.

9. Placement of one and one-half inch  $(1 \frac{1}{2})$  asphalt coarse (MT, 9.5mm) on driveways, pads, and county roads/city streets.

10. Placement of granular material on the shoulders as directed to raise the existing shoulders to the new surface course grade.

NOTE: Shoulders shall be bladed, shaped and compacted throughout the length of the project regardless of whether granular material is required.

NOTE: Granular material not required for the final shape of the shoulders may require removal under the pay item for excess excavation and may include small amounts of asphalt, excluding granular material resulting from the trench widening operation as described in item 4.

NOTE: Due care shall be taken during this operation to blade material to the roadway and away from the ditch line. Material inadvertently bladed to the roadway vegetation shall be removed at no cost to the Mississippi Department of Transportation.

11. Placement of Temporary Traffic Stripe daily as per Special Provisions Nos. 907-618-4.

12. Install rumble strips.

13. Place permanent pavement markings as required (Thermoplastic Striping, Red-Clear and Two-Way Yellow Reflective High Performance Raised Markers).

### <u>All ramps are to be paved a maximum distance of 10' or to the existing ROW and shall be</u> tapered to fit existing conditions to allow smooth entry and exit. County roads and/or city streets are to be paved and/or milled to the right-of-way line. County roads and/or city streets are to be restriped.</u>

The contractor shall provide all signs and traffic handling devices necessary to safely maintain traffic around or through the work areas.

Incidental work such as removing vegetation, shaping and compaction of shoulder, necessary and incidental grading of roadway ditches and other incidental work that is necessary to complete the work will not be measured for separate payment and the cost will be included in the bid items provided.

MDOT Maintenance forces will remove the existing signs located at the taper transition from four-lane to two-lane in the Westbound lane.

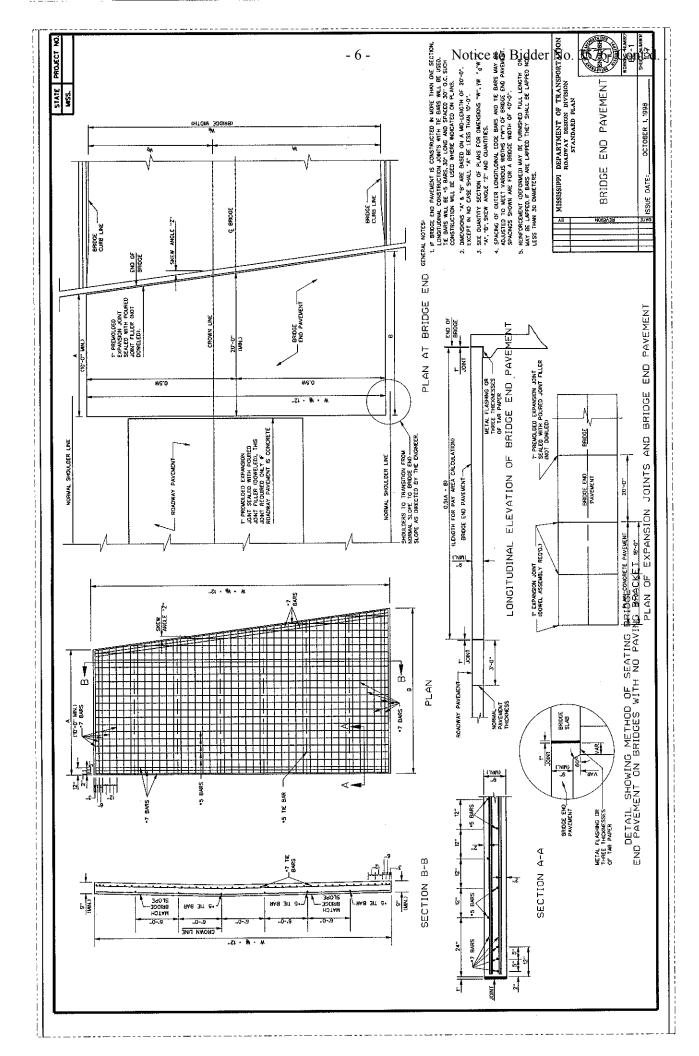
#### MILLING TABLE

Station	Quantity (SF)
0+44.6 EB	13,756.74
0+44.6 WB	6,037.21
106+22.32 EB	18,839.24
106+22.32 WB	19,763.88
152+92.38 EB	36,141.44
241+67.99 WB	11,667.27
269+50.93 EB	28,332.71
269+50.93 WB	46,864.39
320+23.58 WB	6,511.08
346+80.78 WB	6,895.84
361+07.12 EB	24,028.14
491+55.61 EB	25,253.97
491+55.61 WB	43,494.18
533+27.47 EB	34,073.72
533+27.47 WB	7,555.96
Bridge Ends	84,000.00

302	277,418.00
GR PADS	20,920.00
BOP/EOP	7,200.00
Total	approx.=.80,000 SY

The Bridge ends will be milled by milling through the Guardrail pad, then 150' taper to tie to existing. The BOP and EOP will be milled with 150' tapers and tied to existing pavement.

Bridge Approach Slab Repair on US 72									
Bridge Number	Station	Bridge Width (LF)	Holes (Each) 512-A	Grout (Lbs.) 907-512- B	Calcium Chloride (Lbs) 512-C	508-A Silicone Sealed Joints (LF)			
12.2 A	186+52.33	43	4	5000	150	41			
	189+43.13		4	5000	150	41			
12.2 B	187+65.88	43	4	5000	150	41			
	187+65.88		4	5000	150	41			
Total			16	20000	600	164			



## FOR INFORMATION ONLY

#### SPECIAL PROVISION NO. 907-110-2

CODE: (SP)

DATE: 04/02/2010

#### **SUBJECT:** Wage Rates

Section 110, Required Contract Provisions, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

907-110.02--Application. Delete Subsection 110.02.2 on page 100 and substitute the following.

<u>907-110.02.2--Wage Rates.</u> All persons employed or working upon the site of the work will be paid at wage rates not less than those contained in the wage determination decision of the Secretary of Labor in effect 10 days prior to taking bids.

Bidders are advised that regardless of the wage rates listed in the Supplement to FHWA 1273 in the contract, minimum federal wage rates must be paid.

#### SPECIAL PROVISION NO. 907-512-1

#### CODE: (SP)

DATE: 03/6/2005

#### SUBJECT: Grout Mixture

Section 907-512, Pressure Grouting Concrete Pavement, of the 2004 Standard Specifications for Road and Bridge Construction is hereby amended as follows:

<u>**907-512.02.2--Portioning Grout Mixture.</u>** Delete the table at the top of page 342 and substitute the following:</u>

#### GROUT MIXTURES PERCENT BY WEIGHT OF DRY MATERIALS

DRY MATERIALS	TYPES					
	1	2	3	4	5	6
Cement	25	25	25	25	30	25
Limestone Dust		25	75	50		
Fly Ash	25			25	70	
Fine Sand	50	50				75
Calcium Chloride	*	*	*	*	*	
* As prescribed in Subsection 512.03.1						

<u>**907-512-05--Basis of Payment.</u>** Delete the first pay item listed on the top of page 347, and substitute the following:</u>

907-512-B: Portland Cement Pressure Grout Slurry, Type \_\_\_\_\_ - per pound

#### SPECIAL PROVISION NO. 907-701-4

CODE: (IS)

DATE: 11/09/2010

#### SUBJECT: Hydraulic Cement

Section 701, Hydraulic Cement, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

Delete Subsection 701.01 on pages 595 & 596, and substitute the following:

<u>907-701.01--General</u>. The following requirements shall be applicable to hydraulic cement:

Only hydraulic cements conforming to Section 701 shall be used. Hydraulic cements shall not be listed or designated as meeting more than one AASHTO or Department type.

Different brands of hydraulic cement, or the same brand of hydraulic cement from different mills, shall not be mixed or used alternately in any one class of construction or structure, without written permission from the Engineer; except that this requirement will not be applicable to hydraulic cement treatment of design soils, or bases.

The Contractor shall provide suitable means for storing and protecting the hydraulic cement against dampness. Hydraulic cement, which for any reason, has become partially set or which contains lumps of caked hydraulic cement will be rejected. Hydraulic cement salvaged from discarded or used bags shall not be used.

The temperature of bulk hydraulic cement shall not be greater than 165°F at the time of incorporation in the mix.

Acceptance of hydraulic cement will be based on the certification program as described in the Department's Materials Division Inspection, Testing, and Certification Manual and job control sampling and testing as established by Department SOP.

Retests of hydraulic cement may be made for soundness and expansion within 28 days of test failure and, if the hydraulic cement passes, it may be accepted. Hydraulic cement shall not be rejected due to failure to meet the fineness requirements if upon retests after drying at 212°F for one hour, it meets such requirements.

Delete Subsection 701.02 on page 596, and substitute the following:

#### 907-701.02--Portland Cement.

907-701.02.1--General.

**<u>907-701.02.1.1--Types of Portland Cement.</u>** Portland cement (cement) shall be either Type I or Type II conforming to AASHTO Designation: M85 or Type I(MS), as defined by the description below Table 1. Type III cement conforming to AASHTO Designation: M85 or Type III(MS), as defined by the description below Table 1, may be used for the production of precast or precast-prestressed concrete members.

<u>907-701.02.1.2--Alkali Content</u>. All cement types in this Subsection shall meet the Equivalent alkali content requirement for low-alkali cements listed in AASHTO Designation: M85, Table 2.

<u>907-701.02.2--Replacement by Other Cementitious Materials</u>. The maximum replacement of cement by weight is 25% for fly ash or 50% for ground granulated blast furnace slag (GGBFS). The minimum tolerance for replacement shall be 5% below the maximum replacement content. Replacement contents below this minimum tolerance by fly ash or GGBFS may be used, but shall not be given any special considerations, like the maximum acceptance temperature for Portland cement concrete containing pozzolans. Special considerations shall only apply for replacement of cement by fly ash or GGBFS.

<u>907-701.02.2.1--Portland Cement Concrete Exposed to Soluble Sulfate Conditions or</u> <u>Seawater.</u> When Portland cement concrete is exposed to moderate or severe soluble sulfate conditions, or to seawater, cement types and replacement of cement by Class F fly ash, GGBFS, or silica fume shall be as follows in Table 1.

Sulfate Exposure	Water-soluble sulfate (SO <sub>4</sub> ) in soil, % by mass	Sulfate (SO <sub>4</sub> )in water, ppm	Cementitious material required*
Moderate and Seawater	0.10 - 0.20	150 - 1,500	Type II **, ***, **** cement, or Type I cement with one of the following replacements of cement by weight: 25% Class F fly ash, 50% GGBFS, or 8% silica fume
Severe	0.20 - 2.00	1,500 - 10,000	Type I cement with a replacement by weight of 50% GGBFS, or Type II ** cement with one of the following replacements of cement by weight: 25% Class F fly ash, 50% GGBFS, or 8% silica fume

#### Table 1- Cementitious Materials for Soluble Sulfate Conditions

- \* The values listed in this table for replacement of Portland cement by the cementitious materials listed are maximums and shall not be exceeded. The minimum tolerance for replacement shall be 0.5% below the maximum replacement content. Replacement contents below this minimum tolerance by the cementitious materials listed in this table do not meet the requirements for the exposure conditions listed and shall not be allowed.
- \*\* Type I cement conforming to AASHTO Designation: M85 with a maximum 8% tricalcium aluminate ( $C_3A$ ) may be used in lieu of Type II cement; this cement is given the designation "Type I(MS)". Type III cement conforming to AASHTO Designation: M85 with a maximum 8% tricalcium aluminate ( $C_3A$ ) may be used in lieu of Type II cement as allowed in Subsection 907-701.02.1; this cement is given the designation "Type III(MS)".
- \*\*\* Blended cement meeting the sulfate resistance requirements of Subsection 907-701.04 may be used in lieu of Type II as allowed in Subsection 907-701.04. No additional cementitious materials shall be added to or as a replacement for blended cement.
- \*\*\*\* Class F fly ash or GGBFS may be added as a replacement for cement as allowed in Subsection 907-701.02.2.

Class C fly ash shall not be used as a replacement for cement in any of the sulfate exposure conditions listed above.

**907-701.02.2.2--Cement for Soil Stabilization Exposed to Soluble Sulfate Conditions or** <u>Seawater.</u> When Portland cement for use in soil stabilization is exposed to moderate or severe soluble sulfate conditions, or to seawater, cement types and replacement of cement by Class F fly ash or GGBFS shall meet the requirements of Subsection 907-701.02.2.1. Neither metakaolin nor silica fume shall be used to bring the cementitious materials into compliance with the requirements of Table 1.

Delete Subsection 701.03 on page 596, and substitute the following:

<u>**907-701.03--Masonry Cement</u>**. Masonry cement shall conform to ASTM Designation: C 91 and shall only be used in masonry applications.</u>

Delete Subsection 701.04 on page 596, and substitute the following:

#### 907-701.04--Blended Hydraulic Cement.

#### 907-701.04.1--General.

<u>**907-701.04.1.1--Types of Blended Cement.</u>** Blended hydraulic cements (blended cements) shall be of the following types and conform to AASHTO Designation: M 240:</u>

Type I(SM)–Slag-modified Portland cementType IS–Portland blast-furnace slag cementType I(PM)–Pozzolan-modified Portland cementType IP–Portland-pozzolan cement

Blended cement for use in Portland cement concrete or soil stabilization exposed to the moderate soluble sulfate condition or exposure to seawater as defined in Table 1 shall meet the Sulfate resistance requirement listed in AASHTO Designation: M 240, Table 2 and the "(MS)" suffix shall be added to the type designation.

<u>907-701.04.1.2--Alkali Content.</u> All blended cement types in this Subsection shall meet the Mortar expansion requirements listed in AASHTO Designation: M 240, Table 2.

<u>907-701.04.2--Replacement by Other Cementitious Materials</u>. No additional cementitious materials, such as Portland cement, performance hydraulic cement, fly ash, GGBFS, metakaolin, or others, shall be added to or as a replacement for blended cement.

<u>907-701.04.3--Exposure to Soluble Sulfate Conditions or Seawater.</u> When Portland cement concrete or blended cement for soil stabilization is exposed to moderate soluble sulfate conditions or to seawater, where the moderate soluble sulfate condition is defined in Table 1, the blended cement shall meet the sulfate resistance requirement listed in AASHTO Designation: M 240, Table 2.

When Portland cement concrete or blended cement for soil stabilization is exposed to severe soluble sulfate conditions, where the severe soluble sulfate condition is defined in Table 1, blended cements shall not be used.

#### SPECIAL PROVISION NO. 907-711-4

CODE: (IS)

#### DATE: 06/26/2009

#### SUBJECT: Synthetic Structural Fiber Reinforcement

Section 711, Reinforcement and Wire Rope, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

After Subsection 711.03.4.3 on page 665, add the following:

<u>907-711.04--Synthetic Structural Fiber.</u> The synthetic structural fibers shall be approved for listing in the Department's "Approved Sources of Materials" prior to use. The synthetic structural fibers shall be added to the concrete and mixed in accordance with the manufacturer's recommended methods.

<u>907-711.04.1--Material Properties.</u> The fibers shall meet the requirements of ASTM Designation: C 1116, Section 4.1.3. The fibers shall be made of polypropylene, polypropylene/polyethylene blend, nylon, or polyvinyl alcohol (PVA).

<u>907-711.04.2--Minimum Dosage Rate.</u> The dosage rate shall be such that the average residual strength ratio ( $R_{150,3,0}$ ) of fiber reinforced concrete beams is a minimum of 20.0 percent when the beams are tested in accordance with ASTM Designation: C 1609. The dosage rate for fibers shall be determined by the following.

The fiber manufacturer shall have the fibers tested by an acceptable, independent laboratory acceptable to the Department and regularly inspected by the Cement and Concrete Reference Laboratory of the National Institutes of Standards and Technology and approved to perform ASTM Designations: C 39, C 78, and C192.

The laboratory shall test the fibers following the requirements of ASTM Designation: C 1609 in a minimum of three (3) test specimens cast from the same batch of concrete, molded in 6 x 6 x 20-inch standard beam molds meeting the requirements of ASTM Designation: C 31. The beams shall be tested on an 18-inch span. The tests for  $R_{150,3.0}$  shall be performed when the average compressive strength of concrete used to cast the beams is between 3500 and 4500 psi. The tests for compressive strength shall follow the requirements of ASTM Designation: C 39. The average compressive strength shall be determined from a minimum of two (2) compressive strength cylinders.

The value for  $R_{150,3}$  shall be determined using the following equation:

$$R_{150,3.0} = \frac{f_{150,3.0}}{f_1} \times 100$$

The residual flexural strength  $(f_{150,3,0})$  shall be determined using the following equation:

$$f_{150,3.0} = \frac{P_{150,3.0} \times L}{b \times d^2}$$

where:

 $f_{150,3,0}$  is the residual flexural strength at the midspan deflection of L/150, (psi),

 $P_{150,3,0}$  is the residual load capacity at the midspan deflection of L/150, (lbf),

L is the span, (in),

b is the width of the specimen at the fracture, (in), and

*d* is the depth of the specimen at the fracture, (in).

For a 6 x 6 x 20-inch beam, the  $P_{150,3,0}$  shall be measured at a midspan deflection of 0.12 inch.

Additionally,  $R_{150,3.0}$ ,  $f_{150,3.0}$ , and  $P_{150,3.0}$  may also be referred to as  $R_{150}^{150}$ ,  $f_{150}^{150}$ , and  $P_{150}^{150}$  respectively.

At the dosage rate required to achieve the minimum  $R_{150,3}$ , the mixture shall both be workable and the fibers shall not form clumps.

The manufacturer shall submit to the State Materials Engineer certified test reports from the independent laboratory showing the test results of each test specimen.

<u>907-711.04.3--Job Control Requirements.</u> The synthetic structural fibers shall be one from the Department's "Approved Sources of Materials."

At the required dosage rate, the mixture shall both be workable and the fibers shall not form clumps to the satisfaction of the Engineer. If the mixture is determined by the Engineer to not be workable or have clumps of fibers, the mixture may be rejected.

#### SPECIAL PROVISION NO. 907-713-2

CODE: (IS)

#### DATE: 11/09/2010

#### **SUBJECT:** Admixtures for Concrete

Section 713, Concrete Curing Materials and Admixtures, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

After the second paragraph of Subsection 713.01.2 on page 676, add the following.

Type 1-D compound may be used on bridge rails, median barriers, and other structures requiring a spray finish. When Type 1-D compound is used, it will be the Contractor's responsibility to assure that the compound has dissipated from the structure prior to applying the spray finish and that the spray finish adheres soundly to the structure.

Delete Subsection 713.02 on pages 676 & 677, and substitute the following:

<u>907-713.02--Admixtures for Concrete</u>. Air-entraining admixtures used in Portland cement concrete shall comply with AASHTO Designation: M 154. Set-retarding, accelerating, and/or water-reducing admixtures shall comply with AASHTO Designation: M 194. Water-reducing admixture shall meet the minimum requirements for Type A. Set-retarding admixtures shall meet the minimum requirements for Type D.

In order to obtain approval of an admixture, the State Materials Engineer shall have been furnished certified test reports, made by an acceptable independent laboratory regularly inspected by the Cement and Concrete Reference Laboratory of the National Institutes of Standards and Technology, which show that the admixture meets all the requirements of the applicable AASHTO Standard Specification.

The Department reserves the right to sample, for check tests, any shipment or lot of admixture delivered to a project.

The Department reserves the right to require tests of the material to be furnished, using the specific cement and aggregates proposed for use on the project, as suggested in AASHTO Designation: M 154 and outlined in AASHTO Designation: M 194.

After an admixture has been approved, the Contractor shall submit to the State Materials Engineer, with each new lot of material shipped, a certification from the manufacturer in accordance with the requirements of Subsection 700.05.1 and stating the material is of the same composition as originally approved and has not been changed or altered in any way. The requirement in Subsection 700.05.1(b) is not required on the certification from the manufacturer.

Admixtures containing chlorides will not be permitted.

Failure to maintain compliance with any requirement of these specifications shall be cause for rejection of any previously approved source or brand of admixture.

Admixtures shall only be used in accordance with the manufacturer's recommended dosage range as set forth in the manufacturer's approval request correspondence. When an admixture is used in Portland cement concrete, it shall be the responsibility of the Contractor to produce satisfactory results.

<u>907-713.02.1--Source Approval.</u> In order to obtain approval of an admixture, the Producer/Suppliers shall submit to the State Materials Engineer the following for review: certified test reports, made by an acceptable independent laboratory regularly inspected by the Cement and Concrete Reference Laboratory of the National Institutes of Standards and Technology, which show that the admixture meets all the requirements of the applicable AASHTO or Department Specification for the specific type and the dosage range for the specific type of admixture.

907-713.02.2--Specific Requirements. Admixtures containing chlorides will not be permitted.

<u>907-713.02.3--Acceptance.</u> The Department reserves the right to sample, for check tests, any shipment or lot of admixture delivered to a project.

The Department reserves the right to require tests of the material to be furnished, using the specific cement and aggregates proposed for use on the project, as suggested in AASHTO Designation: M 154 and outlined in AASHTO Designation: M 194.

Failure to maintain compliance with any requirement of these specifications shall be cause for rejection of any previously approved source or brand of admixture.

With each new lot of material shipped the Contractor shall submit to the State Materials Engineer, a notarized certification from the manufacturer showing that the material complies with the requirements of the applicable AASHTO or Department Specification.

When an admixture is used, it shall be the responsibility of the Contractor to produce satisfactory results.

#### SPECIAL PROVISION NO. 907-714-6

CODE: (IS)

DATE: 11/09/2010

#### **SUBJECT:** Miscellaneous Materials

Section 714, Miscellaneous Materials, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

<u>907-714.05--Fly Ash</u>. Delete Subsections 714.05.1 & 714.05.2 on pages 680 & 681, and substitute the following:

<u>907-714.05.1--General.</u> The fly ash source must be approved for listing in the Department's "Approved Sources of Materials" prior to use. The acceptance of fly ash shall be based on certified test reports, certification of shipment from the supplier, and tests performed on samples obtained after delivery in accordance with the Department's Materials Division Inspection, Testing, and Certification Manual and Department SOP.

Different classes of fly ash or different sources of the same class shall not be mixed or used in the construction of a structure or unit of a structure without written permission from the Engineer.

The Contractor shall provide suitable means for storing and protecting the fly ash from dampness. Separate storage silos, bins, or containers shall be provided for fly ash. Fly ash which has become partially set or contains lumps of caked fly ash shall not be used.

The temperature of the bulk fly ash shall not be greater than 165°F at the time of incorporation into the work.

All classes of fly ash shall meet the supplementary option chemical requirement for available alkalies listed in AASHTO Designation: M 295, Table 2. Class F fly ash shall have a calcium oxide (CaO) content of less than 6.0%. Class C fly ash shall have a CaO content of greater than or equal to 6.0%.

The replacement of Portland cement with fly ash shall be in accordance with the applicable replacement content specified in Subsection 907-701.02.2.

In addition to these requirements, fly ash shall meet the following specific requirements for the intended use.

<u>907-714.05.2--Fly Ash for Use in Concrete</u>. When used with Portland cement in the production of concrete or grout, the fly ash shall meet the requirements of AASHTO Designation: M 295, Class C or F, with the following exception:

The loss on ignition shall not exceed 6.0 percent.

No additional cementitious materials, such as blended hydraulic cement, GGBFS, metakaolin, or others, shall be added to or as a replacement for Portland cement when used with fly ash.

<u>**907-714.06--Ground Granulated Blast Furnace Slag (GGBFS)**</u>. Delete Subsection 714.06.1 on page 681, and substitute the following:

<u>907-714.06.1--General.</u> The GGBFS source must be approved for listing in the Department's "Approved Sources of Materials" prior to use. The acceptance of GGBFS shall be based on certified test reports, certification of shipment from the supplier, and tests performed on samples obtained after delivery in accordance with the Department's Materials Division Inspection, Testing, and Certification Manual and Department SOP.

The Contractor shall provide suitable means for storing and protecting the GGBFS against dampness and contamination. Separate storage silos, bins, or containers shall be provided for GGBFS. GGBFS which has become partially set, caked or contains lumps shall not be used.

The State Materials Engineer shall be notified in writing of the nature, amount and identity of any processing or other additions made to the GGBFS during production.

GGBFS from different mills shall not be mixed or used alternately in any one class of construction or structure without written permission from the Engineer; except that this requirement will not be applicable to cement treatment of design soils or bases.

No additional cementitious materials, such as blended hydraulic cement, fly ash, metakaolin, or others, shall be added to or as a replacement for Portland cement when used with GGBFS in the production of concrete. The replacement of Portland cement with GGBFS shall be in accordance with the applicable replacement content specified in Subsection 907-701.02.2.

Delete Subsection 714.07 on page 682, and substitute the following:

#### 907-714.07--Additional Cementitious Materials.

#### 907-714.07.1--Metakaolin.

<u>907-714.07.1.1--General.</u> Metakaolin shall only be used as a supplementary cementitious material in Portland cement concrete for compliance with the requirements for cementitious materials exposed to soluble sulfate conditions. Metakaolin from different sources shall not be mixed or used alternately in any one class of construction or structure without written permission from the Engineer. No additional cementitious materials, such as blended hydraulic cement, fly ash, GGBFS, or others, shall be added to or as a replacement for Portland cement when used with metakaolin in the production of concrete.

The State Materials Engineer shall be notified in writing of the nature, amount and identity of any processing, or other additions made to the metakaolin during production.

<u>907-714.07.1.2--Source Approval.</u> The approval of each metakaolin source shall be on a case by case basis as determined by the State Materials Engineer. In order to obtain approval of a metakaolin source, the Producer/Suppliers shall submit to the State Materials Engineer the

following for review: certified test reports, made by an acceptable, independent laboratory regularly inspected by the Cement and Concrete Reference Laboratory of the National Institutes of Standards and Technology, which show that the metakaolin meets all the requirements of AASHTO Designation: M295, including the Effectiveness in contributing to sulfate resistance, Procedure A, listed in AASHTO Designation: M295, Table 4 for Supplementary Optional Physical Requirements, and other requirements listed herein.

In order to demonstrate effectiveness in contributing to sulfate resistance, included in this test data shall be results of metakaolin from the proposed source tested in accordance with ASTM Designation: C 1012. There shall be two sets of test specimens per the following:

- a. One set of test specimens shall be prepared using a Type I Portland cement meeting the requirements of AASHTO Designation: M85 and having a tricalcium aluminate  $(C_3A)$  content of more than 8.0%,
- b. One set of test specimens shall be prepared using a Type II Portland cement meeting the requirements of AASHTO Designation: M85.
- c. The proposed metakaolin shall be incorporated at the rate of 10% cement replacement in each set of test specimens and shall meet both of the acceptance criteria listed below for source approval.

The requirement for acceptance of the test sample using Type I Portland cement is an expansion of 0.10% or less at the end of six months. The requirement for acceptance of the test sample using Type II Portland cement is an expansion of 0.05% or less at the end of six months.

<u>907-714.07.1.3--Storage</u>. The Contractor shall provide suitable means for storing and protecting the metakaolin against dampness and contamination. Metakaolin which has become partially set, caked, or contains lumps shall not be used.

<u>907-714.07.1.4--Specific Requirements</u>. Metakaolin shall meet the requirements of AASHTO Designation: M 295, Class N with the following modifications:

- 1. The sum of  $SiO_2 + Al_2O_3 + Fe_2O_3$  shall be at least 85%. The Material Safety Data Sheet shall indicate that the amount of crystalline silica, as measured by National Institute of Occupation Safety and Health (NIOSH) 7500 method, after removal of the mica interference, is less than 1.0%.
- 2. The loss on ignition shall be less than 3.0%.
- 3. The available alkalies, as equivalent  $Na_2O$ , shall not exceed 1.0%.
- 4. The amount of material retained on a No. 325 mesh sieve shall not exceed 1.0%.
- 5. The strength activity index at seven (7) days shall be at least 85%.

<u>907-714.07.1.5--Acceptance.</u> With each new lot of material shipped the Contractor shall submit to the State Materials Engineer a certified test report from the manufacturer showing that the material meets the requirements AASHTO Designation: M295, Class N and the requirements of this Subsection.

The Department reserves the right to sample, for check tests, any shipment or lot of metakaolin delivered to a project.

#### 907-714.07.2--Silica Fume.

<u>907-714.07.2.1--General.</u> Silica fume shall only be used as a supplementary cementitious material in Portland cement concrete for compliance with the requirements for cementitious materials exposed to soluble sulfate conditions. Silica fume from different sources shall not be mixed or used alternately in any one class of construction or structure without written permission from the Engineer. No additional cementitious materials, such as blended hydraulic cement, performance hydraulic cement, fly ash, GGBFS, or others, shall be added to or as a replacement for Portland cement when used with silica fume in the production of concrete.

The State Materials Engineer shall be notified in writing of the nature, amount and identity of any processing, or other additions made to the silica fume during production.

<u>907-714.07.2.2--Source Approval.</u> The approval of each silica fume source shall be on a case by case basis as determined by the State Materials Engineer. In order to obtain approval of a silica fume source, the Producer/Suppliers shall submit to the State Materials Engineer the following for review: certified test reports, made by an acceptable, independent laboratory regularly inspected by the Cement and Concrete Reference Laboratory of the National Institutes of Standards and Technology, which show that the silica fume meets all the requirements of AASHTO Designation: M307, Table 3, including the Sulfate resistance expansion, listed in the table for Optional Physical Requirements, and other requirements listed herein.

In order to demonstrate effectiveness in contributing to sulfate resistance, included in this test data shall be results of silica fume from the proposed source tested in accordance with ASTM Designation: C 1012. There shall be two sets of test specimens per the following:

- a. One set of test specimens shall be prepared using a Type I Portland cement meeting the requirements of AASHTO Designation: M85 and having a tricalcium aluminate ( $C_3A$ ) content of more than 8.0%,
- b. One set of test specimens shall be prepared using a Type II Portland cement meeting the requirements of AASHTO Designation: M85.
- c. The proposed silica fume shall be incorporated at the rate of 8% cement replacement in each set of test specimens and shall meet both of the acceptance criteria listed below for source approval.

The requirement for acceptance of the test sample using Type I Portland cement is an expansion of 0.10% or less at the end of six months. The requirement for acceptance of the test sample using Type II Portland cement is an expansion of 0.05% or less at the end of six months.

<u>907-714.07.2.3--Storage.</u> The Contractor shall provide suitable means for storing and protecting the silica fume against dampness and contamination. Silica fume which has become partially set, caked, or contains lumps shall not be used.

<u>907-714.07.2.4--Acceptance.</u> With each new lot of material shipped, the Contractor shall submit to the State Materials Engineer a certified test report from the manufacturer showing that the material meets the Chemical and Physical Requirements of AASHTO Designation: M307.

The Department reserves the right to sample, for check tests, any shipment or lot of silica fume

delivered to a project.

Delete Subsection 714.11.6 on pages 690 and 691, and substitute the following:

#### 907-714.11.6--Rapid Setting Cementitious Patching Compounds for Concrete Repair.

Rapid setting concrete patching compounds must be approved for listing in the Department's "Approved Sources of Materials" prior to use. Upon approval, a product must be recertified every four (4) years to remain on the "Approved Sources of Materials" list. Each product shall be pre-measured and packaged dry by the manufacturer. All liquid solutions included by the manufacturer as components of the packaged material shall be packaged in a watertight container. The manufacturer may include aggregates in the packaged material or recommend the addition of Contractor furnished aggregates.

The type, size and quantity of aggregates, if any, to be added at the job site shall be in accordance with the manufacturer's recommendations and shall meet the requirements of Subsection 703.02 for fine aggregate and Subsection 703.03 for coarse aggregate. Required mixing water to be added at the job site shall meet the requirements of Subsection 714.01.2.

Only those bonding agents, if any, recommended by the manufacturer of the grout or patching compounds may be used for increasing the bond to old concrete or mortar surfaces.

Patching compounds containing soluble chlorides will not be permitted when in contact with steel.

Site preparation, proportioning of materials, mixing, placing and curing shall be performed in accordance with the manufacturer's recommendation for the specific type of application, and the Contractor shall furnish a copy of these recommendations to the Engineer.

Rapid setting cementitious concrete patching compounds, including components to be added at the job site, shall conform to the following physical requirements:

Non-shrink cementitious grouts shall not be permitted for use.

Compressive strength shall equal or exceed 3000 psi in 24 hours in accordance with ASTM C 928 for Type R2 concrete or mortar.

Bond strength shall equal or exceed 1000 psi in 24 hours in accordance with ASTM C 928 for Type R2 concrete or mortar.

The material shall have a maximum length change of  $\pm 0.15\%$  in accordance with ASTM C 928 for Type R2 concrete or mortar.

The Contractor shall furnish to the Engineer three copies of the manufacturer's certified test report(s) showing results of all required tests and certification that the material meets the specifications when mixed and place in accordance with the manufacturer's instructions. When the mixture is to be placed in contact with steel, the certification shall further state that the packaged material contains no chlorides. Certified test report(s) and certification shall be furnished for each lot in a shipment.

The proportioning of materials must be approved by the State Materials Engineer and any subsequent change in proportioning must also be approved. A sample of each component shall be submitted to the Engineer along with the quantity or percentage of each to be blended. At least 45 days must be allowed for initial approval.

The proportioning of materials for subsequent lots may be approved by the State Materials Engineer upon receipt of certification from the manufacturer that the new lot of material is the same composition as that originally approved by the Department and that the material has not been changed or altered in any way.

#### **<u>907-714.11.7--Commercial Grout for Anchoring Doweled Tie Bars in Concrete.</u> Before Subsection 714.11.7.1 on page 691, add the following:**

Approved Non-"Fast Set" Epoxy anchor systems as specified below may be used for the repair of concrete pavements that do not involve permanent sustained tension applications or overhead applications.

"*Fast Set Epoxy*" may not be used for any Adhesive Anchor Applications. Adhesive Anchor Systems (Fast Set epoxy or otherwise) shall not be used for permanent sustained tension applications or overhead applications. "Fast Set Epoxy" refers to an epoxy produced by the Sika Corporation called Sikadur AnchorFix-3 and repackaged for sale under a variety of names/companies listed at the Federal Highway Administration web site at the following link:

http://www.fhwa.dot.gov/Bridge/adhesives.cfm

<u>907-714.11.7.4--Acceptance Procedure</u>. After the last sentence of the first paragraph of Subsection 714.11.4 on page 691, add the following:

Upon approval, a product must be recertified every four (4) years to remain on the "Approved Sources of Materials" list.

#### 907-714.11.8--Epoxy Joint Repair System.

**<u>907-714.11.8.1--General.</u>** After the last sentence of the first paragraph of Subsection 714.11.8.1 on page 692, add the following:

Upon approval, a product must be recertified every four (4) years to remain on the "Approved Sources of Materials" list.

#### SPECIAL PROVISION NO. 907-804-13

CODE: (IS)

#### DATE: 11/09/2010

#### **SUBJECT: Concrete Bridges And Structures**

Section 804, Concrete Bridges And Structures, of the 2004 Edition of the Mississippi Standard Specifications for Road and Bridge Construction is hereby amended as follows:

#### 907-804.02-- Materials.

907-804.02.1--General. Delete the third and fourth sentences of the first paragraph of Subsection 804.02.1 on page 846, and substitute the following:

For projects with 1000 cubic yards and more, quality control and acceptance shall be achieved through statistical evaluation of test results. For projects of more than 200 but less than 1000 cubic yards, quality control and acceptance shall be achieved by individual test results.

Add the following materials to the list of materials in Subsection 804.02.1 on page 847.

Blended Cement	
Ground Granulated Blast Furnace Slag (GGBFS)	
Silica Fume	

907-804.02.8--Laboratory Accreditation. In Table 1 of Subsection 804.02.8 on page 849, substitute AASHTO: R 39 - Making and Curing Concrete Test Specimens in the Laboratory for AASHTO: T 126 - Making and Curing Concrete Test Specimens in the Laboratory.

907-804.02.9--Testing Personnel. Delete Table 2 in this subsection and replace it with the following.

	Table 2	Cartificantina Descriptionality
Concrete Technician's Tasks	Test Method Required	Certification Required**
Sampling or Testing of Plastic Concrete	AASHTO Designation:T 23, T 119, T 121, T 141, T 152, T 196, and ASTM Designation: C 1064	MDOT Class I certification
Compressive Strength Testing of Concrete Cylinders	AASHTO Designation: T 22 and T 231	MDOT Concrete Strength Testing Technician certification
Sampling of Aggregates	AASHTO Designation: T 2	Work under the supervision of an MDOT Class II certified technician

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Testing of Aggregates	AASHTO Designation: T 19,	MDOT Class II certification
	T 27, T 84, T 85, T 248, and	
	T 255	
Proportioning of Concrete	AASHTO Designation: M 157	MDOT Class III
Mixtures*	and R 39	
Interpretation and	AASHTO Designation: T 325	MDOT Class III or Two
Application of Maturity	and ASTM Designation:	hours maturity method
Meter Readings	C 1074	training

- \* Technicians making concrete test specimens for meeting the requirements of Subsection 804.02.10.1.2 shall be MDOT Class I certified and under the direct supervision of an MDOT Class III certified technician.
- \*\* MDOT Class I certification encompasses the same test procedures and specifications as ACI Concrete Field Testing Technician Grade I. MDOT Class II certification encompasses the same test procedures and specifications as ACI Aggregate Testing Technician Level 1. MDOT Concrete Strength Testing Technician encompasses the same test procedures and specifications as ACI Concrete Strength Testing certification.

For specifics about the requirements for each level of certification, please refer to the latest edition of the Department's *Concrete Field Manual*. Technicians holding current MDOT Class I, MDOT Class II and/or MDOT Class III certifications shall be acceptable until those certifications expire. Upon a current certification expiration, recertification with the certifications listed in Table 2 shall be required. Technicians currently performing either specific gravity testing of aggregates or compressive strength tests shall be required to either:

- have the required MDOT certification listed in Table 2, or
- have a current MDOT Class III certification or work under the direct supervision of current MDOT Class III technician, and have demonstrated the specific gravity and/or compressive strength test during the inspection of laboratory equipment by the Materials Division, Concrete Section.

<u>**907-804.02.10--Portland Cement Concrete Mix Design.** Delete the first sentence of the first paragraph of Subsection 804.02.10 on page 850 and substitute the following:</u>

At least 30 days prior to production of concrete, the Contractor shall submit to the Engineer proposed concrete mixture designs complying with the Department's *Concrete Field Manual*.

Delete the Notes under Table 3 of Subsection 804.02.10 on pages 850 & 851, and substitute the following:

- \* Maximum size aggregate shall conform to the concrete mix design for the specified aggregate.
- \*\* The replacement limits of Portland cement by weight by other cementitious materials (such as fly ash, GGBFS, metakaolin, silica fume, or others) shall be in accordance with the values in Subsection 907-701.02. Other hydraulic cements may be used in accordance with the specifications listed in Section 701.

- \*\*\* The slump may be increased up to eight (8) inches with :
  - an approved water-reducing admixture,
  - an approved water-reducing/set-retarding admixture, or
  - a combination of an approved water-reducing admixture and an approved setretarding admixture, in accordance with 907-713.02. Minus slump requirements shall meet those set forth in Table 3 of AASHTO Designation: M157.
- \*\*\*\* Entrained air is not required except for concrete exposed to seawater. For concrete exposed to seawater, the total air content shall be 3.0 % to 6.0%. For concrete not exposed to seawater, the total air content shall not exceed 6.0%.
- \*\*\*\*\* Class DS Concrete for drilled shafts shall have an 8±1-inch slump.

Delete the last paragraph of Subsection 804.02.10 on page 851 and substitute the following:

At least one water-reducing admixture shall be used in all classes of concrete in accordance with the manufacturer's recommended dosage range. Any combinations of admixtures shall be approved by the Engineer before their use.

**907-804.02.10.1.1--Proportioning on the Basis of Previous Field Experience of Trial** <u>Mixtures.</u> Delete the first sentence of the first paragraph of Subsection 804.02.10.1.1 on page 851, and substitute the following:

Where a concrete production facility has a record, based on at least 10 consecutive strength tests from at least 10 different batches within the past 12 months from a mixture not previously used on Department projects, the standard deviation shall be calculated.

<u>**907-804.02.10.3--Field Verification of Concrete Mix Design.</u> Delete the first sentence of the third paragraph of Subsection 804.02.10.3 on page 853 and substitute the following:</u>** 

For all Classes of concrete, the mixture shall be verified to yield within 2.0% of the correct volume when all the mix water is added to the batch.

For all Classes of concrete other than DS, F, and FX, the mixture shall produce a slump within a minus  $1\frac{1}{2}$ -inch tolerance of the maximum permitted for mixtures with a maximum permitted slump of three inches (3") or less or within a minus  $2\frac{1}{2}$ -inch tolerance of the maximum permitted for mixtures with a maximum permitted slump of greater than three inches (3"), and producing a total air content within a minus  $1\frac{1}{2}$  percent tolerance of the maximum allowable air content in Table 3.

For Class DS, the slump shall be within the requirements in Note \*\*\*\*\* below Table 3. For Class DS exposed to seawater, the total air content shall be within a minus 1½ percent tolerance of the maximum allowable air content in Note \*\*\*\* below Table 3. For Class DS not exposed to seawater the total air content shall be within the requirements in Note \*\*\*\* below Table 3.

For Classes F and FX, the slump shall be within a minus 1<sup>1</sup>/<sub>2</sub>-inch tolerance of the maximum permitted for mixtures with a maximum permitted slump of three inches (3") or less or within a minus 2<sup>1</sup>/<sub>2</sub>-inch tolerance of the maximum permitted for mixtures with a maximum permitted

slump of greater than three inches (3"). For Classes F and FX exposed to seawater, the total air content shall be within a minus 1½ percent tolerance of the maximum allowable air content in Note \*\*\*\* below Table 3. For Classes F and FX not exposed to seawater the total air content shall be within the requirements in Note \*\*\*\* below Table 3.

Delete the third sentence of the third paragraph of Subsection 804.02.10.3 on page 853, and substitute the following:

If the requirements of yield, slump, or total air content are not met within three (3) production days after the first placement, subsequent field verification testing shall not be permitted on department projects, and the mix design shall not be used until the requirements listed above are met

<u>**907-804.02.10.4--Adjustments of Mixture Proportions</u></u>. Delete the paragraph in Subsection 804.02.10.4 on page 854, and substitute the following:</u>** 

The mixture may be adjusted by the Class III Certified Technician representing the Contractor in accordance with the allowable revisions listed in the Department's Concrete Field Manual, paragraph 5.7. Written notification shall be submitted to the Engineer a minimum of seven (7) days prior to any source or brand of material change, aggregate size change, allowable material type change, or decrease in any cementitious material content. Any adjustments of the concrete mixture design shall necessitate repeat of field verification procedure as described in Subsection 804.02.10.3 and approval by the Engineer.

<u>**907-804.02.11--Concrete Batch Plants.</u>** Delete the first three paragraphs of Subsection 804.02.11 on page 854, and substitute the following:</u>

The concrete batch plant shall meet the requirements of the National Ready Mixed Concrete Association *Quality Control Manual, Section 3, Plant Certification Checklist* as outlined in the latest edition of the Department's *Concrete Field Manual*. The Contractor shall submit a copy of the approved checklist along with proof of calibration of batching equipment, i.e., scales, water meter, and admixture dispenser, to the Engineer 30 days prior to the production of concrete.

For projects with 1000 cubic yards and more, the concrete batch plant shall meet the requirements for an automatic system capable of recording batch weights. It shall also have automatic moisture compensation for the fine aggregate. For projects of more than 200 but less than 1000 cubic yards the plant can be equipped for manual batching with a fine aggregate moisture meter visible to the plant operator.

The concrete batch plant shall have available adequate facilities to cool concrete during hot weather.

Mixer trucks to be used on the project are to be listed in the checklist and shall meet the requirements of the checklist.

<u>907-804.02.12--Contractor's Quality Control.</u> Delete the fourth paragraph of Subsection 804.02.12 on page 854 & 855, and substitute the following:

The Contractor's Quality Control program shall encompass the requirements of AASHTO Designation: M 157 into concrete production and control, equipment requirements, testing, and batch ticket information. The requirement of AASHTO Designation: M 157, Section 11.7 shall be followed except, on arrival to the job site, a maximum of 1½ gallons per cubic yard is allowed to be added. Water shall not be added at a later time. If the maximum permitted slump is exceeded after the addition of water at the job site, the concrete shall be rejected.

<u>**907-804.02.12.3--Documentation.**</u> After the second sentence of the second paragraph of Subsection 804.02.12.3 on page 856, add the following:

Batch tickets and gradation data shall be documented in accordance with Department requirements. Batch tickets shall contain all the information in AASHTO Designation: M157, Section 16 including the additional information in Subsection 16.2 with the following exception: the information listed in paragraphs 16.2.7 and 16.2.8 is not required. Batch tickets shall also contain the concrete producer's permanent unique mix number assigned to the concrete mix design.

<u>907-804.02.12.5--Non-Conforming Materials.</u> In Table 4 of Subsection 804.02.12.5 on page 857, delete "/ FM" from the requirements on line B.3.a.

In Table 4 of Subsection 804.02.12.5 on page 857, replace "One set (two cylinders) for 0-100 yd<sup>3</sup> inclusive" with "A minimum of one set (two cylinders) for each 100 yd<sup>3</sup>,"

<u>**907-804.02.13--Quality Assurance Sampling and Testing.</u> Delete subparagraph c) in Subsection 804.02.13 on page 858 and substitute the following:</u>** 

c) For concrete, the Contractor's QC and Department's QA testing of concrete compressive strengths compare when using the data comparison computer program with an alpha value of 0.01 for projects with 1000 cubic yards and more; or, strength comparisons are within 990 psi for projects of more than 200 but less than 1000 cubic yards.

In Table 5 of Subsection 804.02.13 on page 858, delete "and FM" from the requirements on line A.3.

Delete Subsection 907-804.02.13.1 beginning on page 859 and substitute the following:

#### 907-804.02.13.1--Basis of Acceptance.

<u>907-804.02.13.1.1--Sampling</u>. Sampling of concrete mixture shall be performed in accordance with the latest edition of the Department's *Concrete Field Manual*.

**<u>907-804.02.13.1.2--Slump</u>**. Slump of plastic concrete shall meet the requirements of Table 3: MASTER PROPORTION TABLE FOR STRUCTURAL CONCRETE DESIGN. A check test shall be made on another portion of the sample before rejection of any load.

<u>907-804.02.13.1.3--Air</u>. Total air content of concrete shall be within the specified range for the class of concrete listed in Table 3: MASTER PROPORTION TABLE FOR STRUCTURAL CONCRETE DESIGN. A check test shall be made on another portion of the sample before rejection of any load.

<u>907-804.02.13.1.4--Yield</u>. If the yield of the concrete mix design is more than plus or minus 3% of the designed volume, the mix shall be adjusted by a Class III Certified Technician representing the Contractor to yield the correct volume plus or minus three percent ( $\pm$ 3%). If batching of the proportions of the mix design varies outside the batching tolerance range of the originally approved proportions by more than the tolerances allowed in Subsection 804.02.12.1, the new proportions shall be field verified per Subsection 804.02.10.3.

<u>907-804.02.13.1.5--Temperature</u>. Cold weather concreting shall follow the requirements of Subsection 907-804.03.16.1. Hot weather concreting shall follow the requirements of Subsection 804.03.16.2 with a maximum temperature of 95°F for Class DS concrete or for concrete mixes containing cementitious materials meeting the requirements of Subsection 907-701.02.2 as a replacement of Portland cement. For other concrete mixes, the maximum concrete temperature shall be 90°F. Concrete with a temperature more than the maximum allowable temperature shall be rejected and not used in Department work.

<u>907-804.02.13.1.6--Compressive Strength</u>. Laboratory cured concrete compressive strength tests shall conform to the specified strength  $(f'_c)$  listed in the specifications. Concrete represented by compressive strength test below the specified strength  $(f'_c)$  may be removed and replaced by the Contractor. If the Contractor elects not to remove the material, it will be evaluated by the Department as to the adequacy for the use intended. All concrete evaluated as unsatisfactory for the intended use shall be removed and replaced by the Contractor at no additional cost to the Department. For concrete allowed to remain in place, reduction in payment will be as follows:

**Projects with 1000 Cubic Yards and More.** When the evaluation indicates that the work may remain in place, a statistical analysis will be made of the QC and QA concrete test results. If this statistical analysis indicates at least 93% of the material would be expected to have a compressive strength equal to or greater than the specified strength  $(f_c)$  and 99.87% of the material would be expected to have a compressive strength at least one standard deviation above the allowable design stress  $(f_c)$ , the work will be accepted. If the statistical analysis indicates that either of the two criteria are not met, the Engineer will provide for an adjustment in pay as follows for the material represented by the test result.

Total Pay on Material in Question = Unit Price - (Unit Price x % Reduction)

% Reduction = 
$$\frac{(f'_c - X)}{f'_c - (f_c + s)} \times 100$$

where:

 $f_c$  = Specified 28-day compressive strength, psi

- X = Individual compressive strength below  $f'_c$ , psi
- s =standard deviation, psi\*
- $f_c$  = allowable design stress, psi
- \* Standard deviation used in the above reduction of pay formula shall be calculated from the applicable preceding compressive strengths test results plus the individual compressive strength below  $f'_c$ . If below  $f'_c$  strengths occur during the project's first ten compressive strength tests, the standard deviation shall be calculated from the first ten compressive strength tests results.

**Projects of More Than 200 but Less Than 1000 Cubic Yards.** When the evaluation indicates that the work may remain in place, a percent reduction in pay will be assessed based on a comparison of the deficient 28-day test result to the specified strength. The Engineer will provide for an adjustment in pay as follows for the material represented by the test result.

Total Pay on Material in Question = Unit Price - (Unit Price x % Reduction)

% Reduction = 
$$\frac{(f'_c - X)}{f'_c} \times 100$$

where:

 $f'_c$  = Specified 28-day compressive strength, psi X = Individual compressive strength below  $f'_c$ , psi

#### 907-804.03--Construction Requirements.

#### 907-804.03.6--Handling and Placing Concrete.

**<u>907-804.03.6.2--Consolidation.</u>** After the last sentence of Subsection 804.03.6.2 on page 864, add the following:

If the Department determines that there is an excessive number of projections, swells, ridges, depressions, waves, voids, holes, honeycombs or other defects in the completed structure, removal of the entire structure may be required as set out in Subsection 105.12.

<u>907-804.03.15--Removal of Falsework, Forms, and Housing</u>. Delete the first sentence of the second paragraph of Subsection 804.03.15 on page 871, and substitute the following:

Concrete in the last pour of a continuous superstructure shall have attained a compressive strength of 2,400 psi, as determined by cylinder tests or maturity meter probe, prior to striking any falsework.

Delete the first sentence of the third paragraph of Subsection 804.03.15 on page 871, and substitute the following:

At the Contractor's option and with the approval of the Engineer, the time for removal of forms may be determined by cylinder tests, in accordance with the requirements listed in Table 6, in which case the Contractor shall furnish facilities for testing the cylinders.

Delete the fourth and fifth paragraphs of Subsection 804.03.15 on pages 871 & 872, and substitute the following:

The cylinders shall be cured under conditions which are not more favorable than those existing for the portions of the structure which they represent.

Delete the table in Subsection 804.03.15 on page 872, and substitute the following:

#### Table 6 Minimum Compressive Strength Requirements for Form Removal

#### Forms:

1000 psi
1000 psi
1000 psi
2000 psi
2000 psi
2400 psi
1000 psi

#### **Centering:**

Under Beams	2400 psi
Under Bent Caps	2000 psi

#### **Limitation for Placing Beams on:**

Pile Bents, pile under beam	2000 psi
Frame Bents, two or more columns	2200 psi
Frame Bents, single column	2400 psi

In lieu of using concrete strength cylinders to determine when falsework, forms, and housings can be removed, an approved maturity meter may be used to determine concrete strengths by inserting probes into concrete placed in a structure. The minimum number of maturity meter probes required for each structural component shall be in accordance with Table 7. Falsework, forms, and housings may be removed when maturity meter readings indicate that the required concrete strength is achieved. Procedures for using the maturity meter and developing the strength/maturity relationship shall follow the requirements of AASHTO Designation: T 325 and ASTM Designation: C 1074 specifications. Technicians using the maturity meter or calculating strength/maturity graphs shall be required to have at least two hours of training prior to using the maturity equipment.

Structure Component	Quantity of Concrete	No. of Probes
Slabs, beams, walls, & miscellaneous items	$0 - 30 \text{ yd}^3$	2
	> 30 to 60 yd <sup>3</sup>	3
	$> 60 \text{ to } 90 \text{ yd}^3$ $> 90 \text{ yd}^3$	4
	$> 90 \text{ yd}^3$	5
Footings, Columns & Caps	$0 - 13 \text{ yd}^3$	2
	$> 13 \text{ yd}^3$	3
Pavement, Pavement Overlays	$1200 \text{ yd}^2$	2
Pavement Repairs	Per repair or 900 yd <sup>2</sup>	2
	Whichever is smaller	

Table 7Requirements for use of Maturity Meter Probes

#### 907-804.03.16--Cold or Hot Weather Concreting.

<u>907-804.03.16.1--Cold Weather Concreting.</u> After the third paragraph of Subsection 804.03.16.1 on page 873, add the following:

In lieu of the protection and curing of concrete in cold weather, at the option of the Contractor with the approval of the Engineer, when concrete is placed during cold weather and there is a probability of ambient temperatures lower that 40°F, an approved maturity meter may be used to determine concrete strengths by inserting probes into concrete placed in a structure. The minimum number of maturity meter probes required for each structural component shall be in accordance with Table 7. An approved insulating blanketing material shall be used to protect the work when ambient temperatures are less than 40°F and shall remain in place until the required concrete strength in Table 6 is achieved. Procedures for using the maturity meter and developing the strength/maturity relationship shall follow the requirements of AASHTO Designation: T 325 and ASTM Designation: C 1074 specifications. Technicians using the maturity meter or calculating strength/maturity graphs shall be required to have at least two hours of training prior to using the maturity equipment.

Rename the Table in Subsection 804.03.16.1 on page 874 from "Table 6" to "Table 8".

#### 907-804.03.19--Finishing Concrete Surfaces.

#### 907-804.03.19.7--Finishing Bridge Floors.

<u>**907-804.03.19.7.4--Acceptance Procedure for Bridge Deck Smoothness.</u>** After the first sentence of the second paragraph of Subsection 804.03.19.7.4 on page 886, add the following:</u>

Auxiliary lanes, tapers, shoulders and other areas that are not checked with the profilograph, shall meet a 1/8 inch in 10-foot straightedge check made transversely and longitudinally across the deck or slab.

907-804.05--Basis of Payment. Add the "907" prefix to the pay items listed on page 898.

Section 905 Proposal (Sheet 2 - 1)

NH-0007-01(076) / 106159301 Marshall County

Widen and Overlay approximately 12 miles of US 72 from beginning of 4-lane to Benton County line, known as Federal Aid Project No. NH-0007-01(076) / 106159301, in Marshall County.

I (We) agree to complete the entire project within the specified contract time.

# **\*\*\*** SPECIAL NOTICE TO BIDDERS **\*\*\*** BIDS WILL NOT BE CONSIDERED UNLESS BOTH UNIT PRICES AND ITEM TOTALS ARE ENTERED. BIDS WILL NOT BE CONSIDERED UNLESS THE BID CERTIFICATION LOCATED AT THE END OF THE BID SHEETS IS SIGNED \*\*\*BID SCHEDULE\*\*\*

ıt	Ct									
Item Amount	Dollar									
	Ct									
Unit Price	Dollar									
Description		Roadway Items	Removal of Traffic Stripe	Removal of Pavement, All Types and Depths	Excess Excavation, FM, AH	Cold Milling of Bituminous Pavement, All Depths	Rumble Strips, Ground In	Saw Cut, Full Depth	Silicone Sealed Joints	Holes
Units			1,944 Linear Feet	300 Square Yard	300 Cubic Yard	80,000 Square Yard	42 Mile	200 Linear Feet	164 Linear Feet	16 Each
Quantity			1,944	300	300	80,000	42	200	164	16
Adj	Code				(E)					
Item Code			202-B076	202-B078	203-G003	406-A001	423-A001	0052 503-C007 Added 07/18/2011	0053 508-A001 Added 07/18/2011	0055 512-A001 Added 07/18/2011
Line	No.		0010	0020	0030	0040	0050	0052 Added	0053 Added	0055 Added

Section 905 Proposal (Sheet 2 - 2)

	unt			00									
	Bid Amount			10.									
			XXX	00						XXX			
	Unit Price		XXXXXXXX	10.						XXXXXXXX			
	Description	Calcium Chloride	Lump Sum Maintenance of Traffic	Additional Construction Signs	Temporary Traffic Stripe, Continuous White	Temporary Traffic Stripe, Continuous Yellow	Temporary Traffic Stripe, Skip White	Temporary Traffic Stripe, Detail	Temporary Traffic Stripe, Legend	n Mobilization	Red-Clear Reflective High Performance Raised Markers	Two-Way Yellow Reflective High Performance Raised Markers	Granular Material, Class 3, Group D
	Units	Pounds	Lump Sun	Square Feet	Mile	Mile	24 Mile	Linear Feet	Linear Feet	Lump Sum Mobili	Each	300 Each	Ton
	Quantity	600	-	-	28	28	24	59,310	1,235	1	2,500	300	38,400 Ton
	Adj Code												(GT )
	Item Code	0057 512-C002 Added 07/18/2011	618-A001	618-B001	0080 619-A1002 Changed 07/18/2011	0090 619-A2002 Changed 07/18/2011	0100 619-A3006 Changed 07/18/2011	0110 619-A5001 Changed 07/18/2011	619-A6001	620-A001	627-K001	627-L001	907-304-B009
-	Line No.	0057 Added	0900	0070	0080 Change	0090 Change	0100 Change	0110 Change	0120	0130	0140	0150	0160

Section 905 Proposal (Sheet 2 - 3)

	Description Unit Price Bid Amount		Portland Cement Pressure Grout Slurry, Type 1	6" Thermoplastic Traffic Stripe, Skip White	6" Thermoplastic Edge Stripe, Continuous White	6" Thermoplastic Traffic Stripe, Skip Yellow	6" Thermoplastic Traffic Stripe, Continuous Yellow	6" Thermoplastic Edge Stripe, Continuous Yellow	Thermoplastic Detail Stripe, White	Thermoplastic Detail Stripe, Yellow	Thermoplastic Legend, White	ALTERNATE GROUP AA NUMBER 1	
	Units	llon Asphalt for Tack Coat				ar	ar		ır			7	
	Quantity I	78,100 Gallon	20,000 Pounds	24 Mile	28 Mile	703 Line Feet	16,364 Line Feet	24 Mile	28,846 Line: Feet	30,464 Linear Feet	1,235 Linear Feet		
	Adj Code	(A2)											
(c - z march mendar r	Item Code	0170 907-407-A001 Changed 07/18/2011	0175 907-512-B001 Added 07/18/2011	907-626-A003	907-626-C004	907-626-D004	907-626-E003	907-626-F004	907-626-G004	907-626-G005	907-626-H004		
mendot t	Line No.	0170 Change	0175 Added	0180	0190	0200	0210	0220	0230	0240	0250		

Section 905 Proposal (Sheet 2 - 4)

				XXX						XXX			
Bid Amount				XXXXXXXX XXX						xxxxxxx xxx			
<b>A</b>										XXX			
Unit Price				XXXXXXXX						XXXXXXXX			
Description	Hot Mix Asphalt, HT, 4.75-mm mixture	Hot Mix Asphalt, ST, 19-mm mixture, Trench Widening	Hot Mix Asphalt, HT, 9.5-mm mixture, Polymer Modified		ALTERNATE GROUP AA NUMBER 2	Warm Mix Asphalt, MT, 9.5-mm mixture	Warm Mix Asphalt, HT, 4.75-mm mixture	Warm Mix Asphalt, ST, 19-mm mixture, Trench Widening	Warm Mix Asphalt, HT, 9.5-mm mixture, Polymer Modified		ALTERNATE GROUP BB NUMBER 1	6" High Performance Cold Plastic Traffic Stripe, Skip White	6" High Performance Cold Plastic Traffic Stripe, Continuous White
Units	Ton	Ton	Ton			Ton	Ton	Ton	Ton			648 Linear Feet	648 Linear Feet
Quantity	7,700	8,050 Ton	34,900			6,630 Ton	7,700	8,050	34,900 Ton			648	648
Adj Code	(BA1)	(BA1)	(BA1)	(BA1)		(BA1)	(BA1)	(BA1)	(BA1)	(BA1)			
Item Code	265 907-403-A004 Added 07/18/2011	907-403-C005 (BA1)	907-403-D004 (BA1)	0290 907-403-E003 Deleted 07/18/2011		907-403-M006 (BA1)	)305 907-403-M015 (BA1 ) Added 07/18/2011	907-403-0001 (BA1)	907-403-P001 (BA1)	)330 907-403-Q004 (BA1) Deleted 07/18/2011		628-1002	628-J002
Line No.	0265 Added	0270	0280	0290 Delete		0300	0305 Added	0310	0320	0330 Delete		0340	0350

Section 905 Proposal (Sheet 2 - 5)

ınt					
Bid Amount					
Unit Price					
Description	6" High Performance Cold Plastic Traffic Stripe, Continuous Yellow	ALTERNATE GROUP BB NUMBER 2	6" Inverted Profile Thermoplastic Traffic Stripe, Skip White	6" Inverted Profile Thermoplastic Traffic Stripe, Continuous White	6" Inverted Profile Thermoplastic Traffic Stripe, Continuous Yellow
Units	648 Linear Feet		648 Linear Feet	648 Linear Feet	648 Linear Feet
Quantity	648		648	648	648
Adj Code					,
Item Code	0360 628-M002		0370 907-626-1003	907-626-J003	0390 907-626-L001
Line No.	0360		0370	0380	0390

Section 905 Proposal (Sheet 2 - 6)	NH-0007-01(076) / 106159301 Marshall County	1 ty
	*** BID CERTIFICATION ***	
TOTAL BID	\$	
Complete item nos. 1, 2, and/or 3 as appropriate. See	*** DBE/WBE SECTION *** r 3 as appropriate. See Notice to Bidders addressing Disadvantaged Business Enterprises in Highway Construction.	
1. I/We agree that no less than economically disadvantaged	I/We agree that no less than percent shall be expended with small business concerns owned and controlled by socially and economically disadvantaged individuals (DBE and WBE).	
2. Classification of Bidder: Small Business (DBE)_	Small Business (DBE)Small Business (WBE)	
3. A joint venture with a Small Business (DBE/WBE):	all Business (DBE/WBE):	
BIDDER ACKNOWLEDGES THAT HE/SHE HAS CHECKED THEREIN CONSTITUTE THEIR OFFICIAL BID.	*** SIGNATURE STATEMENT *** HE HAS CHECKED ALL ITEMS IN THIS PROPOSAL FOR ACCURACY AND CERTIFIED THAT THE FIGURES SHOWN AL BID.	
	BIDDER'S SIGNATURE	
	BIDDER'S COMPANY	
	BIDDER'S FEDERAL TAX ID NUMBER	

(Date Printed 07/18/11 10:17 am) (Addendum No. 1)